

CEP. BRINDISI

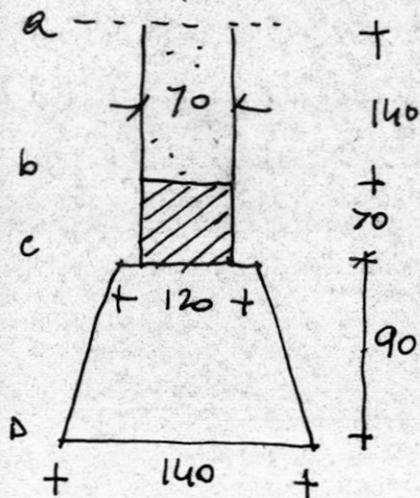
Collo no 9

Edificio 4°.

Ing. VITO GIORGIO COLAIANNI

1

Consorzio Ravennate
della Coop. di Produzione e Lavoro



Verifica della fondazione
continua:

Determinazione della
armatura della cassa
in calcestruzzo armato.

Carico su un metro di fondazione:

$$\text{Solaio } \left(\frac{5.50}{2} + \frac{5.00}{2} \right) \times 600 \times 4 = 12600 \text{ kg/m}$$

$$\text{muro: } 3.20 \times 1800 \times 0.43 = 2477$$

$$3.20 \times 1800 \times 0.50 = 2880$$

$$3.20 \times 1800 \times 0.50 = 2880$$

$$\underline{20837 \text{ kg/m.}}$$

Fondazione: muro terra entro terra

$$0.70 \times 1.40 \times 1800 = 1750 \text{ kg/m}$$

$$\text{Cover: } 0.70 \times 0.70 \times 2500 = 1225 \text{ ''}$$

$$\text{Fondaz.} \quad \underline{2925 \text{ ''}}$$

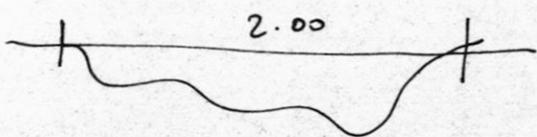
$$5900 \text{ kg/m}$$

Carico totale sul terreno: 26737 a m lineare.

$$\text{Carico a cm. quadro } \frac{26737}{100 \times 1400} = 1.90 \text{ kg/cm}^2$$

Considerando un edificio su una luce di m 2. m luo

(2)



Carico $\approx 24000 \text{ kg/m}$

momento

$$\frac{1}{8} q l^2 = \frac{24000 \times 4}{8} = 12000$$

$$12000 \text{ kg m} = 1200000 \text{ kg cm}$$

$$b = 70 \quad m/b = 17142 \quad \sqrt{\quad} = 132 \quad \delta f = 1400$$

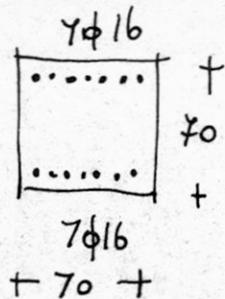
$$\sigma_c = 35 \quad H = 70 \quad h = 68 \quad 68/132 = 0.515$$

D.A. $F_f = F'_f$ simmetrica:

$$0.00153 \times 70 \times 132 = 14.13 \text{ cm}^2$$

$$7 \phi 16 \text{ sopra} = 14.07$$

$$7 \phi 16 \text{ sotto} = 14.07$$



Taglio $R = \frac{24000 \times 2}{2} = 24000$

$$0.92 b h = 4379$$

$$\tau = \frac{R}{2} = 5.48 < 6$$

Adottiamo staffe $\phi 8$ cinese

ogni 20 cm.

$$\tau_{am} = \frac{1400 \times 1.01}{20 \times 70} = \frac{1400}{1400} = 1 \text{ kg/cm}^2$$

$$\tau_{residuo} = 4.48 \text{ kg/cm}^2 \text{ (calcestruzzo) -}$$

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