2/b

FABBRICATO B

CALCOLO DEI PILASTRI

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Ing.

Pilastri 1 e 3 (d'angolo)

- Terrazzo	3.40 × 0.52	=	1.780	Ton	
- Parapetto	$\frac{4.60+3.10}{2}\times 0.250\times 0.80$	=	0.770	"	$A_c = 30\times30 = 900$ cmq.
- Trave	$\frac{4.30+3.10}{2}\times 0.300$	=	1.110	"	$A_f = 4\phi 14 = 6.15$ "
- Pilastro	0.250 × 3.40	=	0.870	"	$A_i = 941.5$ "
			4.530	Ton	$\sigma_c = \frac{4.530}{961.5} = \underline{4.70}$ Kg./cmq.

Carico sul 3° Piano —— 4.530 Ton.

- Solaio	3.00 × 0.500	=	1.500	Ton.	
- Muratura	3.55 × 3.00 × 0.300	=	3.200	"	$A_c = 35\times35 = 1225$ cmq.
- Trave	3.55 × 0.438	=	1.560	"	$A_f = 4\phi 16 = 8.08$ "
- Pilastro	0.310 × 3.40	=	1.060	"	$A_i = 1305.8$ "
			7.320	Ton	$\sigma_c = \frac{11.850}{1305.8} = \underline{9.1}$ Kg./cmq.

Carico sul 2° Piano —— 11.850 Ton.

- Solaio	c.s.	=	1.500	Ton	
- Trave	" "	=	3.200	"	$A_c = 35\times35 = 1225$ cmq
- Muratura	" "	=	1.560	"	$A_f = 4\phi 16 = 8.08$ "
- Pilastro	0.310 × 3.40	=	1.060	"	$A_i = 1305.8$ "
			7.320	Ton	$\sigma_c \frac{19.170}{1305.8} = \underline{14.7}$ Kg/cmq.

Carico sul 1° Piano —— 19.170 Ton.

- Solaio	c.s.	=	1.500	Ton	
- Trave	"	=	3.200	"	$A_c = 35\times35 = 1225$ cmq.
- Muratura	"	=	1.560	"	$A_f = 4\phi 18 = 10.18$ "
- Pilastro	0.310 × 3.40	=	1.060	"	$A_i = 1326.8$ "
			7.320	Ton	$\sigma_c = \frac{26.490}{1326.8} = \underline{20}$ Kg/cmq

Carico sul Piano Terreno —— 26.490 Ton.

Solaio	c.s.	=	1.500	Ton	
Trave	"	=	3.200	"	$A_c = 35\times35 = 1225$ cmq
Muratura	"	=	1.560	"	$A_f = 4\phi 18 = 1018$ "
Pilastro	0.310 × 2.60	=	0.810	"	$A_i = 1326.8$
			7.070	Ton.	$\sigma_c = \frac{33.560}{1326.8} = \underline{25.3}$ Kg/cmq.

Carico sul Piano Fondaz. —— 33.560 Ton.

Pilastro 2

- Terrazzo $3.10 \times \frac{4.30}{2} \times 0,52 = 3.450$ Ton. $A_c = 30 \times 30 = 900$ cmq.
- Parapetto $3.10 \times 0,80 \times 0,25 = 0,620$ " $A_f = 4 \phi 14 = 6.16$ "
- Trave $2.80 \times 0,300 = 0,840$ " $A_i = 941.6$ "
- Pilastro $0,200 \times 3,40 = 0,780$ " $\sigma_c = \frac{5.690}{961.6} = 5,3$ Kg/cmq.

5,690 Ton.

Carico sul 3° Piano ———— 5,69 Ton.

- Solaio $6.70 \times 0.500 = 3,350$ Ton.
- Muratura $2.80 \times 0,300 \times 3,00 = 2,560$ " $A_c. = 30 \times 35 = 1050$ cmq.
- Trave $0,350 \times 2.80 = 0,970$ " $A_f. = 4 \phi 16 = 8,04$ "
- Pilastro $0,250 \times 3,40 = 0,850$ " $A_i = 1130,4$ "

7.730 Ton. $\sigma_c = \frac{13.420}{1130.4} = 12$ Kg/cmq.

Carico sul 2° Piano ———— 13.42 Ton.

- Solaio c. s. = 3.350 Ton.
- Muratura c. s. = 2.560 " $A_c. = 30 \times 35 = 1050$ cmq.
- Trave c. s. = 0,970 " $A_f. = 4 \phi 16 = 8,08$ "
- Pilastro $0.250 \times 3.40 = 0.850$ " $A_i = 1130.4$ "

7.730 Ton. $\sigma_c = \frac{21.150}{1130.4} = 18,7$ Kg/cmq.

Carico sul 1° Piano ———— 21.15 Ton

- Solaio c. s. = 3,350 Ton.
- Muratura " = 2.560 " $A_c. = 35 \times 35 = 1225$ cmq.
- Trave " = 0.970 " $A_f. = 4 \phi 18 = 10.18$ "
- Pilastro " = 1,020 " $A_c. = 1326.8$ "

7.900 Ton. $\sigma_c = \frac{29.050}{1326,8} = 22$ Kg/cmq.

Carico sul Piano Terreno ———— 29.05 Ton.

- Solaio, murat. e trave c.s. = 6.880 Ton $A_c. = 35 \times 35 = 1225$ cmq.
- Pilastro $0.263 \times 2.60 = 0.690$ " $A_f. = 4 \phi 18 = 1018$ "

$A_i. = 1326.8$ "

7.570 Ton.

Carico sul Piano Fond. ———— 36.72 Ton. $\sigma_c = \frac{36.620}{1326.8} = 27.5$ Kg/cmq.

Pilastri 4 e 5 (d'angolo)

- Terrazzo $1.40 \times 1.60 \times 0.520 = 1.165$ Ton
- Parapetto $\frac{1.50+3.50}{2} \times 0.250 = 0,650$ " $A_c = 30 \times 30 = 900$ cmq.
- Travi $\frac{1.20+2.80+3.20}{2} \times 0.300 = 1.860$ " $A_f = 4 \phi 14 = 6.15$ "
- Pilastro $0.250 \times 3.40 = 0.870$ " $A_i = 961.5$ "

Carico sul 3° Piano ———— 4.545 Ton. $\sigma_c = \frac{4545}{961.50} = 4.25$ Kg/cmq.

- Solaio $\frac{2.75}{2} \times \frac{3.15}{2} \times 0.500 = 1.080$ Ton
- Muratura $\frac{1.15+3.15}{2} \times 0,300 \times 3,00 = 1,940$ " $A_c. = 35 \times 35 = 1225$ cmq.
- Travi $\frac{1.15+3.15+2.75}{2} \times 0.350 = 1.230$ " $A_f. = 4 \phi 16 = 8,08$ "
- Balcone $\frac{1.15}{2} \times \frac{3.15}{2} \times 0.750 = 0.670$ " $A_i = 1305.8$ "
- Pilastro $0,310 \times 3,40 = 1,060$ " $\sigma_c = \frac{10.525}{1305.8} = 8.10$ Kg/cmq.

Carico sul 2° Piano ———— 10.525 Ton.

- Solaio, muratura, travi e $A_c. = 35 \times 35 = 1225$ cmq.
- balcone c.s. = 4.920 Ton. $A_f. = 4 \phi 16 = 8.08$ "
- Pilastro c. s. = 1.060 " $A_i = 1305.8$ "

Carico sul 1° Piano ———— 16.505 Ton. $\sigma_c = \frac{16.505}{1306,8} = 12.7$ Kg/cmq.

- Solaio, muratura, travi e $A_c = 35 \times 35 = 1225$ cmq.

balcone c.s. = 4.920 Ton $A_f. = 4 \phi 18 = 10.18$ "

- Pilastro c.s. = 1.060 " $A_i = 1326.8$ "

Carico sul P. Terreno ———— 22.485 Ton. $\sigma_c = \frac{22.485}{1326.8} = 17.0$ Kg/cmq.

- Solaio, muratura, travi c.s. 4.250 Ton. $A_c = 35 \times 35 = 1225$ cmq.
- Pilastro $0.310 \times 2.60 = 0.805$ " $A_f. = 4 \phi 18 = 10.18$ "

Carico sul P.no Fondaz.ni ———— 27.540 Ton $A_i = 1326.8$ "

$\sigma_c = \frac{27.540}{1326.8} = 21.5$ Kg/cmq

Pilastro 6

- Terrazzo $\frac{2.80+3.50}{2}\times\frac{5.00}{2}\times 0.520 =$ 4.100 Ton
- Parapetto $\frac{3.10+3.80}{2}\times 0.250\times 0.80 =$ 0.690 " — $A_c = 30\times30 = 900$ cmq.
- Travi $\frac{2.80+3.50}{2}\times 0.300 =$ 0.945 " — $A_f = 4\phi14 = 6.16$ "
- Pilastro $0.250\times3.40 =$ 0.850 " — $A_i = 961.6$ "

Carico sul 3° Piano ———— 6.585 Ton. $\sigma_c = \frac{6.585}{961.6} = \underline{6.7}$ Kg/cmq.

- Solaio $\frac{2.75+3.45}{2}\times\frac{4.95}{2}\times 0.500 =$ 3.840 Ton.
- Muratura $\frac{2.75\times3.45}{2}\times 0.300\times 3.00 =$ 2.790 " — $A_c = 30\times35 = 1050$ cmq.
- Travi $\frac{2.75+3.45}{2}\times 0.438 =$ 1.360 " — $A_f = 4\phi16 = 8.04$ "
- Pilastro $0.270\times3.40 =$ 0.930 " — $A_i = 1130.4$ "

Carico sul 2° Piano ———— 15.505 Ton. $\sigma_c = \frac{15.505}{1130.4} = \underline{13.70}$ Kg/cmq.

- Solaio, muratura, travi c.s. = 7.990 Ton — $A_c = 30\times35 = 1050$ cmq.
- Pilastro c.s. = 0.930 " — $A_f = 4\phi16 = 8.04$ "

Carico sul 1° Piano ———— 24.425 Ton. $A_i = 1130.4$

$\sigma_c = \frac{24.425}{1130.4} = \underline{21.60}$ Kg/cmq.

- Solaio, muratura, travi c.s. = 7.990 Ton — $A_c = 35\times35 = 1225$ cmq.
- Pilastro 0.310×3.40 = 1.060 " — $A_f = 4\phi18 = 10.18$ "

Carico sul P.no Terr.no ———— 33.475 Ton. $A_i = 1326.8$ "

$\sigma_c = \frac{33.475}{1326.8} = \underline{25.2}$ Kg/cmq.

- Solaio, muratura, travi c.s. = 7.990 Ton — $A_c = 35\times35 = 1225$ cmq.
- Pilastro 0.310×2.60 = 0.810 " — $A_f = 4\phi20 + 2\phi16 = 16.59$

Carico sul P.no Fond.ni ———— 42.275 Ton. $A_i = 1390.9$

$\sigma_c = \frac{42.275}{1390.9} = \underline{30.40}$ Kg/cmq.

Pilastro 7

- Copertura Torrino Scala $\frac{2.85\times5.30}{4}\times 0.400 =$ 1.500 Ton.
- Solaio terrazzo $\frac{3.50}{2}\times\frac{5.00}{2}\times 0.520 =$ 2.280 " — $A_c = 30\times40 = 1200$ cmq.
- Travi $\frac{1}{2}(3.50+2.55)\times 0.375 =$ 1.140 " — $A_f = 2\phi14 + 4\phi16 = 11.12$
- Trave rampante $(1.60+1.55)\times 0.375$ 1.180 " — $A_i = 1311.2$ cmq.
- Scale $(1.60+1.55)\times1.25\times0.750 =$ 2.940 "
- Muratura $(\frac{2.55}{2}\times4.40 + \frac{3.50}{2}\times$
  $\times2.70 + 3.15\times\frac{2.70+4.40}{2})\times 0.300 =$ 6.450 " — $\sigma_c = \frac{16.510}{1311.2} = \underline{12.6}$ Kg/cmq.
- Pilastro $0.300\times3.40 =$ 1.020 "

Carico sul 3° Piano ———— 16.510 Ton.

- Solaio $\frac{3.45}{2}\times\frac{4.95}{2}\times 0.500$ 2.140 Tonn.
- Muratura $\frac{1}{2}(2.55+5.00)\times 0.300\times 2.90 =$ 3.380 " — $A_c = 30\times45 = 1200$ cmq
- Travi $\frac{1}{2}(3.45+2.55)\times 0.438$ = 1.320 " — $A_f = 6\phi16 = 12.06$ "
- Trave rampante $(1.60+1.55)\times 0.375 =$ 1.180 " — $A_i = 1470.6$ "
- Scala $(1.60+1.55)\times1.25\times0.750 =$ 2.940 "
- Pilastro $0.300\times3.40 =$ 1.020 " — $\sigma_c = \frac{28.490}{1470.6} = \underline{19.30}$ Kg/cmq

Carico sul 2° Piano ———— 28.490 Ton.

- Solaio, muratura, travi, trave rampante e Scala c.s. = 10.960 Ton — $A_c = 30\times50 = 1500$ cmq. — $A_f = 4\phi18 + 2\phi16 = 14.2$
- Pilastro 0.300×3.40 = 1.020 " — $A_i = 1642$

Carico sul 1° Piano ———— 40.470 Ton.

$\sigma_c = \frac{40.470}{1642} = \underline{24.6}$ Kg/cmq.

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- Solaio, muratura, travi, trave — $A_c = 30 \times 55 = 1650$ cmq.

  rampante e Scala c.s. = 10.960 Ton. — $A_f = 4\phi 20 + 2\phi 18 = 17.66$ "

- Pilastro 0.375 x 3.40 = 1.260 " — $A_i = 1826$ "

Carico sul P.no Terr.no ———— 52.69 Ton. $\sigma_c = \frac{52.690}{1826} = \underline{28.8}$ Kg/cmq.

- Solaio. muratura. travi. trave — $A_c = 30 \times 60 = 1800$ cmq.

  rampante e Scala c.s. = 10.960 Ton — $A_f = 4\phi 20 + 2\phi 18 = 17.66$ "

- Pilastro 0,400 x 2,60 = 1.040 " — $A_i = 1976.6$ "

Carico sul P.no Fondaz. ———— 64.69 Ton. $\sigma_c = \frac{64.690}{1976.6} = \underline{32.8}$ Kg/cmq.

Pilastro 8

- Copertura Lavatoi e Torrino Scale

  $\frac{5.40}{2} \times \frac{5.30}{2} \times 0.400 =$ 2.860 Ton.

- Solaio terrazzo $\frac{2.70}{2} \times \frac{5.00}{2} \times 0.520 = 1.760$ " — $A_c = 30 \times 40 = 1200$ cmq.
- Sorracc. Lavatoi $\frac{1}{4} \times 2.50 \times 5.30 \times 0.500 = 1.660$ " — $A_f = 2\phi 14 + 4\phi 16 = 11.12$
- Travi $\frac{2.55 + 2.70 + 5.00}{2} \times 0.375 = 1.730$ " — $A_i = 1311.2$ cmq.
- Trave rampante $(1.60 + 1.55) \times 0.375 = 1.180$ "
- Scale $(1.60 + 1.55) \times 0.750 \times 1.25 = 2.940$ " — $\sigma_c = \frac{19.220}{1311.2} = \underline{14.7}$ Kg/cmq.
- Muratura $\left(\frac{2.70}{2} \times 2.70 + \frac{2.55}{2} \times\right.$

  $\left.\times 4.40 + \frac{5.00}{2} \times 4.40\right) \times 0.300 = 6.070$ "
- Pilastro 0,300 x 3.40 = 1.020 "

Carico sul 3° Piano ———— 19.220 Ton.

- Solaio $\frac{2.70}{2} \times \frac{4.95}{2} \times 0.500 = 1.670$ Ton
- Travi $\frac{2.55 + 2.70 + 5.00}{2} \times 0.438 = 2.010$ " — $A_c = 30 \times 45 = 1200$ cmq.
- Trave rampante $(1.60 + 1.55) \times 0.375 = 1.180$ " — $A_f = 6\phi 16 = 12.06$ "
- Scala $(1.60 + 1.55) \times 1.25 \times 0.750 = 2.940$ " — $A_i = 1470.6$ "
- Muratura $\frac{1}{2}(2.70 + 2.55 + 5.00) \times 0.3 \times 2.9 = 4.020$ "
- Pilastro 0.300 x 2.40 = 1.020 " — $\sigma_c = \frac{32.160}{1470.6} = \underline{21.8}$ Kg/cmq.

Carico sul 2° Piano ———— 36.160 Ton.

- Solaio. travi, trave rampante, — $A_c = 30 \times 50 = 1500$ cmq.

  Scala e muratura c.s. = 11.820 Ton. — $A_f = 4\phi 18 + 2\phi 16 = 14.2$

- Pilastro 0.375 x 3.40 = 1.260 " — $A_i = 1642$ cmq.

Carico sul 1° Piano ———— 45.240 Ton.

$\sigma_c = \frac{45.240}{1642} = \underline{27.6}$ Kg/cmq.

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- Solaio, trari, trave rampante, murature e Scale c.s. = 11.820 Ton. $A_c = 30 \times 55 = 1650$ cmq.
- Pilastro c.s. = 1.260 " $A_f = 4\phi 20 + 2\phi 18 = 17.66$
$A_i = 1826$ cmq.

Carico sul P. Terr.no ———— 58.32 Ton.

$$\sigma_c = \frac{58.320}{1826} = \underline{32}\ \text{Kg/cmq.}$$

- Solaio, trari, trave rampante, murature e Scale c.s. = 11.820 Ton. $A_c = 30 \times 60 = 1800$ cmq.
- Pilastro 0.400×2.60 = 1.040 " $A_f = 4\phi 20 + 2\phi 18 = 17.66$
$A_i = 1976.6$ cmq.

Carico sul P.no Fondaz.ni ———— 71.18 Ton.

$$\sigma_c = \frac{71.180}{1976.6} = \underline{36.3}\ \text{Kg/cmq.}$$

Pilastro 9

- Copertura lavatoi $\frac{2.50}{2} \times \frac{5.30}{2} \times 0.400 = 1.375$ Ton.
- Muratura $0.200 \times \frac{2.50+5.30}{2} \times 2.60 = 2.100$ "
- Parapetto $\frac{3.05+4.00}{2} \times 0.250 \times 0.800 = 0.705$ " $A_c = 30 \times 35 = 1050$ cmq.
- Solaio terrazza $\frac{6.45}{2} \times \frac{5.00}{2} \times 0.520 = 4.280$ " $A_f = 4\phi 16 = 8.04$ "
- Sovraccar. Lavatoi $\frac{1}{4} \times 2.50 \times 5.30 \times 0.50 = 1.660$ " $A_i = 1130.4$ "
- Pilastro $0.310 \times 3.40 =$ 1.060 "
- Trari $0.375 \times \frac{2.70+3.70}{2} =$ 1.200 "

$$\sigma_c = \frac{12.380}{1130.4} = \underline{10.9}\ \text{Kg/cmq.}$$

Carico sul 3° Piano ———— 12.380 Ton.

- Solaio $\frac{6.70}{2} \times \frac{5.00}{2} \times 0.500$ = 4.180 Ton $A_c = 35 \times 35 = 1225$ cmq.
- Trari $0.438 \times \frac{6.70}{2}$ = 1.465 " $A_f = 6\phi 16 = 12.06$ "
- Muratura $0.300 \times \frac{6.70}{2} \times 2.90$ = 2.920 " $A_i = 1345.6$ "
- Pilastro $0.310 \times 3.40 =$ 1.060 "

$$\sigma_c = \frac{22.005}{1345.6} = \underline{16.4}\ \text{Kg/cmq.}$$

Carico sul 2° Piano ———— 22.005 Ton.

- Solaio, travi e murature c.s. = 8.565 Ton. $A_c = 35 \times 35 = 1225$ cmq.
- Pilastro 0.310×3.40 = 1.060 " $A_f = 6\phi 16 = 12.06$ "
$A_i = 1345.6$ "

Carico sul 1° Piano ———— 31.630 Ton. $\sigma_c = \frac{31.630}{1345.6} = \underline{23.5}$ Kg/cmq.

- Solaio, travi e murature c.s. = 8.565 Ton $A_c = 35 \times 35 = 1225$ cmq.
- Pilastro c.s. = 1.060 " $A_f = 4\phi 18 + 2\phi 16 = 14.20$ "
$A_i = 1367$ "

Carico sul P.no Terr. ———— 41.255 Ton. $\sigma_c = \frac{41.255}{1367} = \underline{30.2}$ Kg/cmq.

- Solaio, travi e murature c.s. = 8.565 Ton $A_c = 35 \times 40 = 1400$ cmq.
- Pilastro 0.400×2.60 = 1.000 " $A_f = 4\phi 18 + 2\phi 16 = 14.20$ "
$A_i = 1542$ cmq.

Carico sul P. Fondaz. ———— 50.820 $\sigma_c = \frac{50.820}{1542} = \underline{33.-}$ Kg/cmq.

Pilastro 10 (d'angolo)

- Parapetto (2.00+1.50) × 0.250 = 0.880 Ton.
- Terrazzo $\frac{3.70}{2} \times 2.70 \times 0.520$ = 2.600 "
- Travi $\left.\begin{matrix}\frac{2.70}{2} \times 0.300 \\ \frac{3.70}{2} \times 0.375\end{matrix}\right\}$ = 1.100. "
- Pilastro 0.250 × 3.40 = 0.850 "

Carico sul 3° Piano ——— 5.430 Ton.

A_c = 30×30 = 900 cmq.
A_f = $4\phi 14$ = 6.15 "
A_i = 961.5 "
$\sigma_c = \frac{5.430}{961.5}$ = 5.7 Kg/cmq.

- Solaio $\frac{3.65}{2} \times 2.65 \times 0.500$ = 2.420 Ton.
- Balcone 1.70 × 1.50 × 0.750 = 1.920 "
- Muratura $\frac{3.65+2.65}{2} \times 0.300 \times 3.00$ = 2.830 "
- Travi $\left.\begin{matrix}\frac{3.65}{2} \times 0.438 \\ \frac{2.65}{2} \times 0.350\end{matrix}\right\}$ = 1.270 "
- Pilastro 0.263 × 3.40 = = 0.900 "

9.340 "

Carico sul 2° Piano ——— 14.770 Ton.

A_c = 35×35 = 1225 cmq.
A_f = $4\phi 16$ = 8.08 "
A_i = 1305.8 "
$\sigma_c = \frac{14.770}{1305.8}$ = 11.30 Kg/cmq.

- Solaio - Balcone - Muratura -
- - Travi e Pilastro c.s. = 9.340 Ton.

Carico sul 1° Piano ——— 24.110 Ton.

A_c = 35×35 = 1225 cmq.
A_f = $4\phi 16$ = 8.08 "
A_i = 1305.8 "
$\sigma_c = \frac{24.110}{1305.8}$ = 18.50 Kg/cmq.

- Solaio, muratura, travi c.s. = 6.520 Ton.
- Pilastro 0.307 × 3.40 = 1.050 "

7.570 "

Carico sul P. Terreno ——— 31.680 Ton.

A_c = 35×35 = 1225 cmq.
A_f = $4\phi 18$ = 10.18 "
A_i = 1326.8 "
$\sigma_c = \frac{31.680}{1326.8}$ = 24.= Kg/cmq.

- Solaio, Murature e Travi c.s. = 6.520 Ton.
- Pilastro 0.307 × 2.60 = 0.810 "

7.330 "

Carico sul P. Fondaz. ——— 39.010 Ton.

A_c = 35×35 = 1225 cmq.
A_f = $4\phi 18$ = 10.18 "
A_i = 1326.8 = "
$\sigma_c = \frac{39.010}{1326.8}$ = 29.4 Kg/cmq.

Pilastro 11

- Parapetto 3.00 × 0.250 = 0.750 Ton.
- Travi 3.00 × 0.300 = 0.900 "
- Pilastro 3.40 × 0.250 = 0.850 "

0,250 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilast. 6

$\sigma_c = \frac{2.50}{961.6} = \underline{2.60}$ Kg/cmq.

Carico sul 3° Piano ——— 2.500 Ton.

- Muratura 2.70 × 0.300 × 3.00 = 2.450 Ton
- Travi 2.70 × 0.350 = 0.950 "
- Balcone 1.50 × 3.00 × 0.750 = 3.380 "
- Pilastro 0.263 × 3.40 = 0.900 "

7.680 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilast. 6

$\sigma_c = \frac{10.180}{1130.4} = \underline{9.}=$ Kg/cmq.

Carico sul 2° Piano ——— 10.180 Ton.

- Muratura, travi e balcone c.s. 6.780 Ton
- Pilastro 0.263 × 3.40 = 0.900 "

7.680 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilast. 6

Carico sul 1° Piano ——— 17.860 Ton. $\sigma_c = \frac{17.860}{1130.4} = \underline{15.8}$ Kg/cmq.

- Muratura, travi e balcone c.s. 6.780 Ton
- Pilastro 0.307 × 3.40 = 1.050 "

7.830 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilast. 6

Carico sul P. Terr. ——— 25,690 Ton. $\sigma_c = \frac{25.690}{1316.8} = \underline{19.30}$ Kg/cmq.

- Muratura, travi e solaio c.s. 6.780 Ton
- Pilastro 0.307 × 2.60 = 0.810 "

7.590 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilast. 6

Carico sul P. di Fondaz. ——— 33.28 Ton. $\sigma_c = \frac{33.280}{1390.9} = \underline{24.}=$ Kg/cmq.

Pilastro 12

- Terrazzo $\frac{4.10+5.60}{2} \times \frac{3.80}{2} \times 0,520 = 4.800$ Ton.
- Parapetto 3.70 × 0.80 × 0.250 = 0.740 "
- Travi $\left\{\begin{matrix}3.40 \times 0.300\\1.90 \times 0.375\end{matrix}\right\} =$ 1,730 "
- Pilastro 3.40 × 0.250 = 0.850 "

8.120 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilastro 6

$\sigma_c = \frac{8.120}{961.6} = \underline{7.6}$ Kg/cmq.

Carico sul 3° Piano ——— 8.12 Ton.

- Solaio $\frac{4.05+5.55}{2} \times \frac{3.80}{2} \times 0.500 = 4.560$ Ton.
- Muratura 3.35 × 0.300 × 3.00 = 3,020 "
- Travi $\left\{\begin{matrix}3.35 \times 0.350\\1.90 \times 0.438\end{matrix}\right\} =$ 2.000 "
- Pilastro 0.263 × 3.40 = 0.900 "

10.480 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i.\end{matrix}\right\}$ come Pilastro 6

$\sigma_c = \frac{18.600}{1130.4} = \underline{16.5}$ Kg/cmq.

Carico sul 2° Piano ——— 18.60 Ton.

- Solaio, muratura e travi c.s. 9580 Ton.
- Pilastro 0.263 × 3.40 = 0.900 "

10.480 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilastro 6

Carico sul 1° Piano ——— 29.08 Ton. $\sigma_c = \frac{29.080}{1130.4} = \underline{25.7}$ Kg/cmq.

- Solaio, muratura e travi c.s. 9.580 Ton.
- Pilastro 0.307 × 3.40 = 1.050 "

10,630 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilastro 6

Carico sul P. Terr. ——— 39.71 Ton. $\sigma_c = \frac{39.710}{1326.8} = \underline{29.8}$ Kg/cmq.

- Solaio, muratura e travi c.s. 9.580 Ton.
- Pilastro 0.307 × 2.60 = 0.810 "

10.390 Ton.

$\left.\begin{matrix}A_c.\\A_f.\\A_i\end{matrix}\right\}$ come Pilastro 6

Carico sul P. di Fondaz. ——— 50.10 Ton. $\sigma_c = \frac{50.100}{1390.9} = \underline{36.}=$ Kg/cmq.

Pilastro 13 (d'angolo)

- Terrazzo $\left(\frac{3.80}{2} \times \frac{4.10}{2}\right) \times 0.520 =$ 2.030 Ton.
- Parapetto $(2.05+2.20) \times 0.80 \times 0.250 =$ 0.850 "
- Trave $(1.90+2.05) \times 0.300 =$ 1.170 "
- Pilastro $0.250 \times 3.40 =$ 0.850 "

4.900 Ton.

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come Pilastri 4e5

$\sigma_c = \frac{4.900}{961.5} = \underline{5.1}$ Kg/cmq.

Carico sul 3° Piano ——— 4.90 Ton.

- Solaio $\frac{3.80}{2} \times \frac{4.10}{2} \times 0.500 =$ 1.950 Ton.
- Muratura $3.95 \times 0.300 \times 2.90 =$ 3.450 "
- Trave $3.95 \times 0.350 =$ 1.380 "
- Pilastro $0.310 \times 3.40 =$ 1.060 "

7.840 Ton.

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come Pilastri 4e5

$\sigma_c = \frac{12.740}{1305.8} = \underline{9.8}$ Kg/cmq.

Carico sul 2° Piano ——— 7.840 Ton.

- Solaio, muratura e trave c.s. 6.780 Ton.
- Pilastro $0.310 \times 3.40 =$ 1.060 "

7.840 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_c. \end{array}\right\}$ come Pilastri 4e5

Carico sul 1° Piano ——— 20.580 Ton. $\sigma_c = \frac{20.580}{1305.8} = \underline{15.7}$ Kg/cmq.

- Solaio, muratura e trave c.s. 6.780 Ton
- Pilastro $0.310 \times 3.40 =$ 1.060 "

7.840

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come Pilastri 4e5

Carico sul P. Terr. ——— 28.320 Ton. $\sigma_c = \frac{28.320}{1326.8} = \underline{21.6}$ Kg/cmq.

- Solaio, muratura e trave c.s. 6.780 Ton.
- Pilastro c.s. 1.060 "

7.840 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come Pilastri 4e5

Carico sul P. di Fondaz. ——— 36.160 Ton. $\sigma_c = \frac{36.160}{1326.8} = \underline{27.2}$ Kg/cmq.

Pilastro 14

- Terrazzo $4.65 \times \frac{4.10}{2} \times 0.520 =$ 4.960 Ton.
- Parapetto $4.65 \times 0.80 \times 0.250 =$ 0.930 "
- Trave $\left\{\begin{array}{l} 4.45 \times 0.375 \\ 4.10 \times 0.200 \end{array}\right\} =$ 2.490 "
- Pilastro $0.250 \times 3.40 =$ 0.850 "

9.230 Ton.

$A_c. = 30 \times 30 = 900$ cmq.

$A_f. = 4 \phi 14 = 6.16$ "

$A_i. = 961.6$ "

$\sigma_c = \frac{9.230}{961.6} = \underline{8.7}$ Kg/cmq.

Carico sul 3° Piano ——— 9.23 Ton.

- Solaio $4.65 \times \frac{4.05}{2} \times 0.500 =$ 4.700 Ton
- Muratura $4.35 \times 0.300 \times 2.90 =$ 3.800 "
- Travi $\left\{\begin{array}{l} \frac{4.90}{2} \times 0.525 \\ \frac{3.80}{2} \times 0.438 \\ \frac{4.05}{2} \times 0.200 \end{array}\right\} =$ 2.530 "
- Pilastro $0.263 \times 3.40 =$ 0.900 "

11.930 Ton.

$A_c. = 30 \times 35 = 1050$ cmq.

$A_f. = 4 \phi 16 = 8.04$ "

$A_i. = 1130.4$ "

$\sigma_c = \frac{21.160}{1130.4} = \underline{18.7}$ Kg/cmq.

Carico sul 2° Piano ——— 21.16 Ton.

- Solaio, muratura e travi c.s. 11.030 Ton.
- Pilestro $0.263 \times 3.40 =$ 0.900 "

11.930 "

$A_c. = 30 \times 35 = 1050$ cmq.

$A_f. = 4 \phi 16 = 8.04$ "

$A_i. = 1130.4$ "

Carico sul 1° Piano ——— 33.09 Ton. $\sigma_c = \frac{33.090}{1130.4} = \underline{29.10}$ Kg/cmq.

- Solaio, muratura e travi c.s. 11.030 Ton.
- Pilastro $0.307 \times 3.40 =$ 1.050 "

12.080 Ton.

$A_c. = 35 \times 35 = 1225$ cmq

$A_f. = 4 \phi 18 = 10.18$ "

$A_i. = 1326.8$

Carico sul P. Terr. ——— 45.17 Ton. $\sigma_c = \frac{45.170}{1326.8} = \underline{33.9}$ Kg/cmq.

- Solaio, muratura e travi c.s. 11.030 Ton.
- Pilastro $0.350 \times 2.60 =$ 0.910 "

11.940 Ton.

$A_c. = 40 \times 40 = 1600$ cmq.

$A_f. = 4 \phi 20 + 2 \phi 16 = 16.59$

$A_i. = 1765.9$

Carico sul P. Fondaz. ——— 57.11 Ton. $\sigma_c = \frac{57.110}{1765.9} = \underline{32.5}$ Kg/cmq.

Pilastro 15

- Terrazzo $5.20 \times \frac{4.05}{2} \times 0.520 = 5.480$ Ton
- Parapetto $5.20 \times 0.80 \times 0.250 = 1.040$ "
- Travi $4.90 \times 0.375 = 1.840$ "
- Pilastro $0.250 \times 3.40 = 0.850$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

9.210 Ton.

$\sigma_c = \frac{9.210}{961.6} = 9.6$ Kg/cmq.

Carico sul 3° Piano ——— 9.21 Ton.

- Solaio $5.20 \times \frac{4.05}{2} \times 0.500 = 5.250$ Ton.
- Muratura $4.90 \times 0.300 \times 2.90 = 4.250$ "
- Trave $4.90 \times 0.525 = 2.570$ "
- Pilastro $0.263 \times 3.40 = 0.900$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

12.970 Ton.

$\sigma_c = \frac{22.180}{1130.4} = 19.6$ Kg/cmq.

Carico sul 2° Piano ——— 22.18 Ton.

- Solaio, muratura e trave c.s. 12070 Ton
- Pilastro $0.263 \times 3.40 = 0.900$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

12.970 Ton

Carico sul 1° Piano ——— 35.15 Ton. $\sigma_c = \frac{35.150}{1130.4} = 31.1$ Kg/cmq.

- Solaio, muratura e trave c.s. 12.070 Ton
- Pilastro $0.307 \times 3.40 = 1.050$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

13.120 Ton.

Carico sul P. Terr. ——— 48.27 Ton. $\sigma_c = \frac{48.270}{1326.8} = 36.1$ Kg./cmq.

- Solaio, muratura e trave c.s. 12070 Ton.
- Pilastro $0.400 \times 2.60 = 1.040$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

13.110 Ton.

Carico sul P. Fondaz. ——— 61.38 Ton $\sigma_c = \frac{61.380}{1765.9} = 34.8$ Kg./cmq.

Pilastra 16

- Terrazzo $4.90 \times \frac{4.05}{2} \times 0.520 = 5.150$ Ton.
- Parapetto $4.90 \times 0.250 \times 0.80 = 0.890$ "
- Trave Principale 4.60×0.375
- Trave di Collegam. $\frac{4.05}{2} \times 0.375$ } $= 2.490$ "
- Pilastro $0.250 \times 3.40 = 0.850$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

9.380 Ton.

$\sigma_c = \frac{9.380}{961.6} = 9.8$ Kg./cmq.

Carico sul 3° Piano ——— 9.380 Ton.

- Solai $4.90 \times \frac{4.05}{2} \times 0.500 = 4.960$ Ton.
- Muratura $4.60 \times 0.300 \times 2.90 = 4.000$ "
- Travi $\left\{\begin{array}{l}0.525 \times 2.60\\0.438 \times 2.15\\0.375 \times \frac{4.05}{2}\end{array}\right\} = 3.070$ "
- Pilastro $0.263 \times 3.40 = 0.900$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

12.930 Ton.

$\sigma_c = \frac{22.310}{1130.4} = 19.8$ Kg/cmq.

Carico sul 2° Piano ——— 22.31 Ton.

- Solai, Muratura e Travi c.s. 12.030 Ton
- Pilastro $0.263 \times 3.40 = 0.900$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

12.930 Ton.

Carico sul 1° Piano ——— 35.24 Ton $\sigma_c = \frac{35.240}{1130.4} = 31.2$ Kg/cmq.

- Solai, Muratura e Travi c.s. 12.030 Ton.
- Pilastro $0.307 \times 3.40 = 1.050$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i\end{array}\right\}$ come Pilastro 14

13.080 Ton.

$\sigma_c = \frac{48.320}{1326.8} = 36.3$ Kg/cmq.

Carico sul P.no Terreno ——— 48.32 Ton.

- Solai, Muratura e Travi c.s. 12.080 Ton
- Pilastro $0.400 \times 2.60 = 1.040$ "

$\left.\begin{array}{l}A_c.\\A_f.\\A_i.\end{array}\right\}$ come Pilastro 14

13.070 Ton.

Carico sul P. di Fondaz. ——— 61.39 Ton. $\sigma_c = \frac{61.390}{1765.9} = 34.8$ Kg/cmq.

Pilastri 17e 18

- Solaio terrazzo $\left(\frac{4.40+1.75}{2}\times\frac{4.30}{2}+\right.$
  $\left.+\frac{4.30+5.00}{2}\times\frac{3.50}{2}\right)\times 0.520 =$ 6.900 Ton
- Travi $\frac{1}{2}(4.10\times 0.300+3.50\times 0.75+1.40\times 0.263)=$ 1.460 "
- Pilastro $0.250\times 3.40 =$ 0.850 "

Carico sul 3° Piano ———— 9.210 Ton.

$A_c = 30\times 30 = 900$ cmq.
$A_f = 4\phi 14 = 6.15$ "
$A_i = 961.5$ "
$\sigma_c = \frac{9.210}{961.5} = \underline{9.6}$ Kg./cmq.

- Solaio $\left(\frac{4.40+1.75}{2}\times\frac{4.30}{2}+\frac{4.30+5.00}{2}\times\right.$
  $\left.\times\frac{3.50}{2}\right)\times 0.500$ = 6.650 Ton
- Travi $\frac{1}{2}(4.05\times 0.375+3.45\times 0.438+$
  $+1.40\times 0.220)$ = 1.680 "
- Pilastro $0.310\times 3.40 =$ = 1.060 "

Carico sul 2° Piano ———— 18.60 Ton

$A_c = 35\times 35 = 1225$ cmq.
$A_f = 4\phi 16 = 8.08$ "
$A_i = 1305.8$ "
$\sigma_c = \frac{18.600}{1305.8} = \underline{14.3}$ Kg./cmq.

- Solaio e Travi ecc. c.s. 8.330 Ton
- Pilastro c.s. 1.060 "

Carico sul 1° Piano ———— 27.99 Ton.

$A_c = 35\times 35 = 1225$ cmq.
$A_f = 4\phi 16 = 8.08$ "
$A_i = 1305.8 =$ "
$\sigma_c = \frac{27.990}{1305.8} = \underline{21.4}$ Kg/cmq.

- Solaio. Travi ecc. c.s. 8.330 Ton.
- Pilastro c.s. 1.060 "

Carico sul P.no Terreno ———— 37.380 Ton.

$A_c = 35\times 35 = 1225$ cmq.
$A_f = 4\phi 18 = 10.18$ "
$A_i = 1326.8 =$ "
$\sigma_c = \frac{37.380}{1326.8} = \underline{28.5}$ Kg./cmq.

- Solaio. Travi ecc. c.s. 8.330 Ton
- Pilastro 0.310×2.60 = 0.800 "

Carico al P.no di Fondaz. ———— 46.510 Ton.

$A_c = 35\times 35 = 1225$ cmq.
$A_f = 4\phi 18 = 10.18$ "
$A_i = 1326.8 =$ "
$\sigma_c = \frac{46.510}{1326.8} = \underline{35.0}$ Kg./cmq.

Pilastro 19

- Copertura torrino Scale $\frac{2.85\times 5.30}{4}\times 0.400 =$ 1.500 Ton.
- Solaio Terrazzo $\left(\frac{2.85+3.80}{2}\times\frac{4.10}{2}+\right.$
  $\left.+\frac{3.50}{2}\times\frac{5.00}{2}\right)\times 0.520 =$ 5.760 "
- Travi $\frac{1}{2}(2.55+3.50)\times 0.375 =$ 1.130 "
- Trave rampante $(1.60+1.55)\times 0.375 =$ 1.180 "
- Scale $(1.60+1.55)\times 1.25\times 0.750$ = 2.940 "
- Muratura $\frac{1}{2}(2.55+5.00)\times 0.300\times 2.70 =$ 3.060 "
- Pilastro 0.300×3.40 = 1.020 "

Carico sul 3° Piano ———— 16.590 Ton.

$A_c = 30\times 40 = 1200$ cmq.
$A_f = 2\phi 14+4\phi 16 = 11.12$ "
$A_i = 1311.2$ "
$\sigma_c = \frac{16.590}{1311.2} = \underline{12.6}$ Kg/cmq.

- Solaio $\left(\frac{2.85+3.80}{2}\times\frac{4.05}{2}+\frac{3.45}{2}\times\right.$
  $\left.\times\frac{4.95}{2}\right)\times 0.500 =$ 5.450 Ton
- Muratura $\frac{1}{2}(2.55+5.00)\times 0.300\times 3.00$ 3.400 "
- Travi $\frac{1}{2}(2.55+3.45)\times 0.438 =$ 1.320 "
- Trave rampante $(1.60+1.55)\times 0.375$ 1.180 "
- Scale $(1.60+1.55)\times 1.25\times 0.750 =$ 2.940 "
- Pilastro $0.300\times 3.40 =$ 1.020 "

Carico sul 2° Piano ———— 31.900 Ton.

$A_c = 30\times 45 = 1350$ cmq.
$A_f = 6\phi 16 = 12.06$ "
$A_i = 1470.6$ "
$\sigma_c = \frac{31.900}{1470.6} = \underline{21.7}$ Kg/cmq.

- Solaio, Muratura, Travi, Trave rampante e Scale c.s. 14.290 Ton.
- Pilastro $0.300\times 3.40 =$ 1.020 "

Carico sul 1° Piano ———— 47.210 Ton

$A_c = 30\times 50 = 1500$ cmq.
$A_f = 4\phi 18+2\phi 16 = 14.2$ "
$A_i = 1642$ "
$\sigma_c = \frac{47.210}{1642} = \underline{28.7}$ Kg/cmq.

- Solaio. Muratura, Travi, Trave rampante e Scale c.s. 14.290 Ton.
- Pilastro $0.375\times 3.40 =$ 1.260 "

Carico sul P.no Terreno ———— 62.760 Ton.

$A_c = 30\times 55 = 1650$ cmq.
$A_f = 4\phi 20+2\phi 18 = 17.66$ "
$A_i = 1826$
$\sigma_c = \frac{62.760}{1826} = \underline{34.4}$ Kg/cmq.

- Solaio, muratura, travi, trave
  rampante e scale c.s. 14.290 Ton
- Pilastro 0.400 × 2.60 = 1.040 "

$A_c. = 30 \times 0.60 = 1800$ cmq.
$A_f. = 4\phi 20 + 2\phi 18 = 17.66$ "
$A_i. = 1976.6$ "

$\sigma_c = \frac{78.090}{1976,6} = \underline{39.5}$ Kg/cmq.

Carico sul P.no Fondaz. ——— 78.09 Ton

Pilastro 20

- Copertura lavatoi e Torrino Scale
  $\frac{5.40}{2} \times \frac{5.30}{2} \times 0,400 =$ 2.860 Ton.
- Solaio terrazzo $\left(\frac{2.85+3.80}{2} \times \frac{4.10}{2} + \right.$
  $\left. + \frac{5.00}{2} \times \frac{3.50}{2}\right) \times 0.520 =$ 5.760 "
- Sovracc. lavatoi $\frac{1}{4}$ 2.50 × 5.30 × 0,500 = 1.660 "
- Travi $\frac{2,55+3,50+5.00}{2} \times 0,375 =$ 2.070 "
- Scale $\frac{1}{2}$ 2,55 × 1.55 × 0.750 = 1.480 "
- Muratura $\frac{1}{2}(2,55+5.00) \times 2.70 \times 0,300 =$ 3.050 "
- Pilastro 0,300 × 3.40 = 1.020 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come pilastro 19

$\sigma_c = \frac{17.900}{1311.2} = \underline{13.7}$ Kg/cmq.

Carico sul 3° Piano ——— 17.900 Ton

- Solaio, $\frac{2.85+3.80}{2} \times \frac{4.05}{2} + \frac{4.95}{2} \times$
  $\times \frac{3.45}{2}) \times 0.500 =$ 5,450 Ton
- Muratura $\frac{1}{2}(2.55+5,00) \times 0,300 \times 3.00 =$ 3.400 "
- Travi $\frac{1}{2}(2,55+3.45) \times 0.438 =$ 1.320 "
- Trave rampante (1.60+1,55) × 0,375 = 1.180 "
- Scale (1.60+1.55) × 1.25 × 0.750 = 2.940 "
- Pilastro 0.300 × 3.40 = 1,020 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come pilastro 19

$\sigma_c = \frac{33.210}{1470,6} = \underline{22.6}$ Kg/cmq.

Carico sul 2° Piano ——— 33.210 Ton.

- Solaio, Muratura, Travi, Trave
  rampante e Scale c.s. 15.310 Ton
- Pilastro 0.375 × 3.40 = 1.260 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come pilastro 19

$\sigma_c = \frac{49.780}{1642} = \underline{30.1}$ Kg/cmq.

Carico sul 1° Piano ——— 49.780 Ton.

- Solaio, Muratura, Travi, Trave
  rampante e Scale c.s. 15.310 Ton.
- Pilastro c.s. 1.260 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i. \end{array}\right\}$ come pilastro 19

Carico sul P. Terreno ——— 66.350 Ton. $\sigma_c = \frac{66.350}{1826} = \underline{36.2}$ Kg/cmq.

- Solaio, muratura, travi, trave rampante e Scale c.s. 15.310 Ton
- Pilastro $0.400 \times 2.60 =$ 1.040 "

$\left.\begin{array}{l} A_c. \\ A_f. \\ A_i: \end{array}\right\}$ come Pilastro 19

$\sigma_c = \frac{83.700}{1976.60} = \underline{42.3}$ Kg/cmq.

Carico sul P.no Fondaz. ——— 83.700 Ton.

Pilastro 21

- Copertura lavatoi $\frac{2.50}{2} \times \frac{5.30}{2} \times 0.400 =$ 1.375 Ton.
- Muratura $0.200 \times \frac{2.50+5.30}{2} \times 2.60 =$ 2.055 " $\quad$ $A_c. = 35 \times 40 =$ 1400 cmq.
- Solaio terrazzo $4.70 \times 3.65 \times 0.520 =$ 8.920 " $\quad$ $A_f. = 6\phi16 =$ 120.6 "
- Sovracc. lavatoi $\frac{1}{4} \times 2.50 \times 5.30 \times 0.500$ 1.660 " $\quad$ $A_i. =$ 1520.6 "
- Travi $\frac{3.50}{2} \times 0.500 + \frac{3.80}{2} \times 0.375 + \frac{4.10}{2} \times 0.300 =$ 2.200 "
- Pilastro $0.350 \times 3.40 =$ 1.190 "

$\sigma_c = \frac{17.400}{1520.6} = \underline{11.5}$ Kg/cmq.

Carico sul 3° Piano ——— 17.400 Ton

- Solaio $4.65 \times 3.60 \times 0.500 =$ 8.400 Ton. $\quad$ $A_c. = 35 \times 40 =$ 1400 cmq.
- Travi $\frac{3.45+3.75}{2} \times 0.438 + \frac{4.05}{2} \times 0.300 =$ 2.195 " $\quad$ $A_f. = 4\phi18 + 2\phi16 = 14{,}20$ "
- Pilastro $0.350 \times 3.40 =$ 1.190 " $\quad$ $A_i =$ 1542 "

Carico sul 2° Piano ——— 29.185 Ton. $\quad$ $\sigma_c = \frac{29.185}{1542} = \underline{19.0}$ Kg./cmq.

- Solai e travi c.s. = 10.595 Ton $\quad$ $A_c. = 35 \times 40 =$ 1400 cmq.
- Pilastro $0.400 \times 3.40 =$ 1.360 " $\quad$ $A_f. = 4\phi18 + 2\phi16 = 14.2$ "

$A_i. =$ 1542 "

Carico sul 1° Piano ——— 41.130 Ton. $\quad$ $\sigma_c = \frac{41.130}{1542} = \underline{26.5}$ Kg/cmq.

- Solai e travi c.s. = 10.595 Ton $\quad$ $A_c. = 35 \times 45 =$ 1575 cmq.
- Pilastro c.s. = 1.360 " $\quad$ $A_f. = 4\phi20 + 2\phi16 = 16.59$ "

$A_i. =$ 1740.9

Carico sul P.no Terreno ——— 53.085 Ton. $\quad$ $\sigma_c = \frac{53.085}{1740.9} = \underline{30{,}5}$ Kg/cmq.

- Solai e travi c.s. 10.595 Ton $\quad$ $A_c. = 35 \times 50 =$ 1750 cmq.
- Pilastro c.s. 1.360 " $\quad$ $A_f. = 4\phi20 + 2\phi16 = 16.59$ "

$A_i. =$ 1915,9

Carico sul P. Fondaz. ——— 64.040 Ton. $\quad$ $\sigma_c = \frac{64.040}{1915.9} = \underline{33.5}$ Kg/cmq.