IMPRESA ING. U. PATRIZI

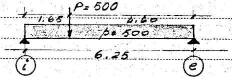
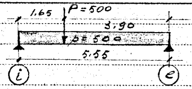
CASE POPOLARI - BRINDISI

Calcoli di stabilità dei fabbr.
A, B e C

Roma, genn. 1956

FABBRICATO "A" 1°, 2° e 3° solaio

Cemento tipo 500
$\sigma = 50 / 1400$; $n = 10$

Solaio	l	p/P		Af/m	t
A	6,25	500/500			
			$m'_i = \frac{1}{8}$ — + —— = 2980 si assume M_i = 2440		
			T_i = 0.62 —— + 370 = 2310 x_e = 2.72		
			T_e = 0.38 — + 130 = 1300 M_m = 1850		
			M h d σ_f R σ_c μ		
			App. int. 2400 × 0.9 = 2200 20.5 18.5 1400 28 50 {0.15 0.51}	{2.76 9.45}	16.7
			Comp. 1850 × 0.9 = 1670 20.5 18.5 1400 – – –	7.1	16.8
B	5,45	500	App. int. Come solaio A		
			Comp. $\frac{1}{14}$ — = 1060 20.5 18.5 1400 – – –	4.35	17.4
C	5.55	500/500			
			$m'_i = \frac{1}{8}$ — + — = 2420 si assume M_i = 1960		
			T_i = 0.627 — + 350 = 2090 x_e = 2.44		
			T_e = 0.373 — + 150 = 1190 M_m = 1490		
			M h d σ_f R σ_c μ		
			App. Int. 1960 × 0.9 = 1760 20.5 18.5 1400 – – –	7.65	17
			Comp. 1490 × 0.9 = 1340 20.5 18.5 1400 – – –	5.6	17
D	4.75	500	App. int. Come solaio adiacente		
			Comp. $\frac{1}{14}$ — = 810 20.5 18.5 1400 – – –	3.3	17.6
E	4.75	500	h d σ_f R σ_c μ		
			Comp. $\frac{1}{11}$ —— = 1030 20.5 18.5 1400 – – –	4.25	17.3
			./.		

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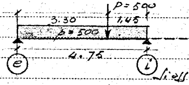
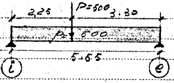
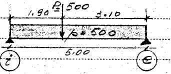
Solaio	l	P/p		Af/m	t
F	5.55	500	M h d σ_f		
			App. int. $\frac{1}{11}$ —— = 1400 20.5 18.5 1400	5.85	17.2
			Comp. $\frac{1}{13}$ —— = 1380 20.5 18.5 1400	4.9	17.2
G	4.75	500/500			
			Si assume M_i = 1400		
			T_e = 0.376 —— + 150 = 1040 x_e = 2.07		
			T_i = 0.624 —— + 350 = 1830 M_m = 1070		
			M h d σ_f		
			App. int. 1400 20.5 18.5 1400	5.85	17.2
			Comp. 1070×1.2 = 1280 20.5 18.5 1400	5.35	17.2
			Comp. bagni 1280 16.5 15.0 1400	6.7	13.7
H	5.55	500/500			
			Si assume M_i = 2000		
			T_i = 0.63 —— + 300 = 2050 x_e = 2.44		
			T_e = 0.37 —— + 200 = 1230 M_m = 1510		
			M h d σ_f		
			App. int. 2000×0.9 = 1800 20.5 18.5 1400	7.6	16.9
			C. bagni al max. x_e = 2.40		
			1500 × 0.9 = 1350 16.5 15 1400	7.1	13.6
I	5.00	500	Comp. $\frac{1}{11}$ —— = 1140 20.5 18.5 1400	4.75	17.3
J	5.00	500/500			
			T_i = 0.5 —— + 310 = 1560		
			x_e = 2.87		
			T_e = 0.5 —— + 190 = 1440		
			M_m = 2070		
			M h d σ_f		
			Comp. 2070×0.8 = 1660 20.5 18.5 1400	7.00	17.00

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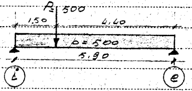
Solaio	l	P/p	
K	5.00	500/500	
			T_i = 0.5 —— + 3
			T_e = 0.5 —— +
			x_e = 3.22
			M_m = 25
			M h d
			Comp. 2590 × 0.8 = 2070 20.5 18.5
			Comp. bagni (x_i max = 1.50) 1000 conven. 16.5 15
L (a sb.)	0.90	650/150	Inc. $\frac{1}{2}$ —— + —— = 400 16.0 14.5
M			16.0
N (a sb.)	1.30	650/150	M h d
			Inc. $\frac{1}{2}$ —— + —— = 740 16.0 14.5

Travi –

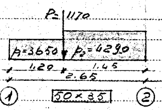
Trave a — Sol. C = 2050
l = 2.65 — " D 0.5×5.00×4.75 = 1190
50×35 — P.pr. (50×35) = 410
p = 3650

T = 0.5× —— = 4840 τ —

M b h d σ_f
App. e Comp. $\frac{1}{12}$ —— = 2140 50 35 32 1400

Trave b

l = 2.65



P) Muro 0.55×500×4.25 =
p_1) Sol. C, D e P.pr. =
p_2) Sol. C = 2050
" G = 1830
P.pr. = 410
p_2 = 4290

	Af/m	t
70 = 1850		
130 = 1610		
90		
σf		
1400	8.85	16.8
1400	5.20	13.8
1400	2.09	13.7
	a.l.	
σf		
1400	3.90	13.6

Af t
5.2 29.5

P = 1170

p_1 = 3650

$p_2 - p_1$ = 640

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$m_1 = \frac{1}{12}$ — $+ \frac{1}{12}$ — $\times 1.29 + 420 = 2710 = M_1$

$m_2 = \frac{1}{12}$ — $+ \frac{1}{12}$ — $\times 2.53 + 350 = 2780 = M_2$

	τ	lo	Af	st.	Ajt	Ap fe
$T_1 = 0.5$ — + — $\times 0.274 + 640 = 5740$	—	—	—	—	—	—
$T_2 = 0.5$ — + — $\times 0.726 + 530 = 6050$	4.1	1.41	10.4	φ6/19"	4.20	6.20

$x_2 = 1.41$ $M_m = -2780 + 4280 = 1500$

	M	b	h	d	σf	Af	t
App.	= 2780	50	35	32	1400	6.8	29.1
Comp.	1500 × 1.5 = 2270	50	35	32	1400	5.5	29.1

Trave -c- come trave a
l. 2.65

Trave d
l = 3.20
50×30

Solaio K = 1850
P.pr. (50×30) = 370
p = 2220

T = 0.5 — = 3550 τ —

	M	b	h	d	σf	Af	t
App. e Comp.	$\frac{1}{12}$ — = 1900	50	30	27	1400	5.3	24.9

Trave e
l = 2.55
50×16

Solaio L (650×0.9+150)1.5 = 1100
P.pr. (50×16) = 200
p = 1300

T. 0.5 — = 1660 τ —

	M	b	h	d	σf	R	σc	M	Af	t
App. e Comp.	$\frac{1}{12}$ — = 710	50	16	14.5	1400	28	50	0.223 0.52	1.64 3.78	13.4

FABBRICATO "A" - Co

Solaio	l.	p.	M	h
A	6.30	600	App. int. $\frac{1}{11}$ — = 2160 Camp. $\frac{1}{13}$ — = 1830	35 35
B	5.50	600	App. verso A $\frac{1}{11}$ — = 2160 Camp. $\frac{1}{13}$ — = 1400	35 35
C	5.60	600	App. int. $\frac{1}{10}$ — = 1880 Camp. $\frac{1}{13}$ — = 1450	35 35
D	4.80	600	App. Int. $\frac{1}{10}$ — = 1880 Camp. $\frac{1}{14}$ — = 990	35 35
E	4.80	600	Camp. $\frac{1}{9}$ — = 1540	35
F	4.90 max	600	Camp. $\frac{1}{8}$ — = 1800	35
G	5.10	600	Camp. $\frac{1}{9}$ — = 1740	35
H	5.90	600	Camp. $\frac{1}{9}$ — = 2320	35
			Copertura scala	
	5.40	400	M	h
			Camp. $\frac{1}{8}$ — = 1460	16.5
			M	h
	4.50		Camp. $\frac{1}{8}$ — = 1020	16.5

opertura

cemento tipo 500

$\sigma = 50/1400$ $n = 10$

d	$\sigma f.$	Af/m	t
33	1400	4.86	31.1
33	1400	4.2	31.1
33	1400	4.86	31.1
33	1400	3.16	31.6
33	1400	4.3	31.3
33	1400	3.31	31.3
33	1400	4.3	31.3
33	1400	2.25	31.5
33	1400	3.52	31.3
33	1400	4.1	31.3
33	1400	3.96	31.3
33	1400	5.35	31.1
d σf R τ			
15 1250 25 50		8.60	136
d σf R σc			
15 1400 — —		5.10	14.2

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Trave d

$l = 2.75$

40×35

Solaio C $0.55 \times 600 \times 5.6 = 1850$

" D $0.55 \times 600 \times 4.8 = 1580$

P.pr. (40×35) = 330

$p = 3760$

	τ	l_0	A_t	st.	Ast.	Ap|τ
$T = 0.5 \frac{}{} = 5270$	4.5	1.37	8.7	ϕ6/22	35	5.2

	M	b	h	d	σf	Af	t
Appoggio	$\frac{1}{15} \frac{}{} = 1900$	40	35	32	1400	4.62	29.5
Camp.	$\frac{1}{11} \frac{}{} = 2590$	(60)	35	32	1400	6.3	29.5

Trave - e

$l = 3.30$

40×35

Solaio H $0.5 \times 600 \times 5.9 = 1770$

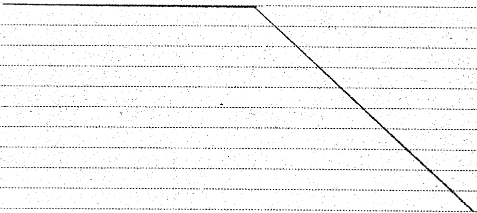
(sol.) = 240

P.pr. (40×40) = 370

$p. = 2380$

$T = 0.5 \frac{}{} = 3940$ $\frac{\tau}{}$

	M	b	h	d	σf	Af	t
Appog.	$\frac{1}{15} \frac{}{} = 1720$	40	35	32	1400	4.15	29.5
Camp.	$\frac{1}{10} \frac{}{} = 2590$	(55)	35	32	1400	6.28	29.5

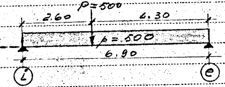


Solaio	l
A	6.9
B	4.9
C a sbalzo	1.3
D	5.7

FABBRICATO B - 1° e 2° solaio

p/p | Af/m | t

500/500



$m'_a = \frac{1}{8}\,\text{—} + \text{—} = 3620$ Si assume $M_a = \underline{\underline{2700}}$

$T_i = 0.614\,\text{—} + 310 = 2430$ $x_e = 3.05$

$T_e = 0.386\,\text{—} + 190 = 1520$ $M_m = 2310$

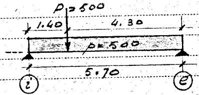
	M	h	d	σ_f	Af/m	t
App. int.	2700 × 0.9 = 2430	23.5	21.5	1400	8.8	19.8
Camp.	2310 × 0.9 = 2080	23.5	21.5	1400	7.5	19.8
Camp. bagni (x_e max)	= 1.70	20.5	18.5	1400	885	16.8

500 App. int. come solaio adiacente

	M	h	d	σ_f	Af/m	t
Camp.	$\frac{1}{14}$ — = 860	23.5	18.5	1400	3.5	17.6
App. int. verso A	= 2430	20.5	18.5	1400	6.3 10.65	16.9

650/150 Incastro $\frac{1}{2}\,\text{—} + \text{—} = 740$ 16.0 18.5 1400 3.9 13.6

500/500



$m'_i = \frac{1}{8}\,\text{—} + \text{—} = 2490$

Si assume $M_i = \underline{\underline{2200}}$

$T_i = 0.636\,\text{—} + 380 = 2190$ $x_e = 2.32$

$T_e = 0.364\,\text{—} + 120 = 1160$ $M_m = 1340$

	M	h	d	σ_f	R	σ_c	μ	Af/m	t
App. int.	= 2200	20.5	18.5	1400	28	50	15 51	2.76 9.45	16.7
Camp.	= 1340	20.5	18.5	1400	—	—	—	6.15	17.1

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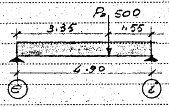
Solaio	l.	p/p		Af/m	t
E	4.90	500/500			



$T_e = 0.5 \times \text{—} + 160 = 1390$ $x_e = 2.79$

$T_i = 0.5 \times \text{—} + 340 = 1570$ $M_m = 1940$

	M	h	d	σ_f	Af/m	t
Comp.	1940 × 0.8 = 1550	20.5	18.5	1400	6.45	17.2
Com. bagni (x_e max)	1.70 =	20.5	18.5	1400	8.45	16.8

Solaio	l.	p/p		h	d	σ_f	Af/m	t
C a sbalzo	1.50 max	650/150	Incastro $\frac{1}{2}\,\text{—} + \text{—} = 960$	16	14.5	1400	5.15	13.4
	1.10 min.	650/150	Incastro $\frac{1}{2}\,\text{—} + \text{—} = 560$	16	14.5	1400	2.95	13.7

Travi

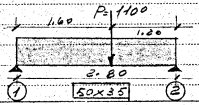
Trave a
l = 3.40
50×40

Solaio B 0.55 × 500 × 4.90 = 1350
Solaio D — = 2190
P.pr. (50×40) = 470
p = 4010

	τ	l_0	Af	St.	Ast.	Ap√2
T = 0.5 × —— = 6800	4.1	1.70	12.3	φ6/19"	5.05	7.25

	M	b	h	d	σ_f	Af	t
App. e Camp.	$\frac{1}{12}$ —— = 3850	50	40	37	1400	8.2	33.6

Trave b



P/Muro 0.5 × 500 × 4.4 = 1100

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p) Solaio B $0.55 \times 500 \times 4.9 = 1350$
Solaio D $= 2190$
P.pr. $(50 \times 35) = 420$
$p = \underline{\underline{3960}}$

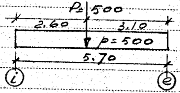
$m'_1 = \frac{1}{12} — + 320 = 2910 = M_1$
$m'_2 = \frac{1}{12} — + 480 = 3020 = M_2$

	τ	l_0	A_t	st	A_{st}	$A_p\sqrt{2}$
$T_1 = 0.496 — + 470 = 5970$	4.1	1.51	11.4	φ6/16"	5.3	6.1
$T_2 = 0.504 — + 630 = 6220$	4.3	1.29	11.4	φ6/16"	4.6	6.8

$x_1 = 1.51$ $M_m = -2910 + 4510 = 1600$

	M	b	h	d	σf	Af	t
App.	= 3020	50	35	32	1400	7.45	29.00
Camp.	1600×1.5 = 2400	50	35	32	1400	5.9	29

Solaio F

Solaio	l	P/p	M	h	d	σf	Af/m	t
F	5.70	500/500	 $m'_i = \frac{1}{8} — + — = 2590$ si assume $M_i = \underline{\underline{1900}}$ $T_i = 0.617 — + 270 = 2030$ $T_e = 0.383 — + 230 = 1320$ $x_e = 2.64$ $M_m = 1750$ App. int. 1900×0.9 = 1710 Camp. bagni 1750×0.8 = 1400	 $\underline{\underline{20.5}}$ $\underline{\underline{16.5}}$	 18.5 15	 1400 1400	 7.2 7.4	 17.0 13.5

FABBRICATO B - Copertura -

Cemento 500 σ = 50/1400 n = 10

Solaio	l	p	M	h	d	σf	Af/m
A	6.80	600	App. int. $\frac{1}{11} —— = 2540$ Camp. $\frac{1}{13} —— = 2140$	$\underline{\underline{35}}$ $\underline{\underline{35}}$	33 33	1400 1400	5.83 4.9
B	5.00	600	App. int. Camp. $\frac{1}{14} —— = 1070$	vedi $\underline{\underline{35}}$	solaio 33	adiacente 1400	 2.44
C	5.70	600	App. int. $\frac{1}{10} —— = 1950$ Camp. $\frac{1}{13} —— = 1500$	$\underline{\underline{35}}$ $\underline{\underline{35}}$	33 33	1400 1400	4.5 3.45
D	4.90	600	Camp. $\frac{1}{9} —— = 1600$	$\underline{\underline{35}}$	33	1400	3.66
E	5.00	600	App. int. vedi solaio adiacente Camp. $\frac{1}{14} —— = 1070$	 $\underline{\underline{35}}$	 33	 1400	 2.44
F	5.00	600	Camp. $\frac{1}{12} —— = 1250$	$\underline{\underline{35}}$	33	1400	2.84
G a sbalzo	1.00	650/150	Incastro $\frac{1}{2} — + — = 330$	$\underline{\underline{10}}$	8.5	1400	3.02
			Solaio copertura scala				
Cop. scala	2.85	400	Camp. $\frac{1}{8} — = 410$	16.5	15	1400	2.06

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t
31.2
31.2
31.4
31.0
31.0
31.3
31.4
31.3
7.8
14.2

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Travi

Trave d

l = 3.40
40×40

Solaio B 0.55×600×5.00 = 1650
" C 0.55×600×5.70 = 1880
P.pr. (40×40) = 370
p = 3900

	τ	l_o	A_t	φt	Ast	$A_p\sqrt{2}$
T = 0.5 —— = 6620	4.9	1.70	11.9	φ6/20"	4.8	7.1

M	b	h	d	σf	Af	t
App. $\frac{1}{15}$ —— = 3000	40	40	37	1400	6.35	33.9
Camp. $\frac{1}{10}$ —— = 4500	(60)	40	37	1400	9.54	33.8

Trave e

l = 2.75
40×35

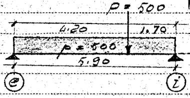
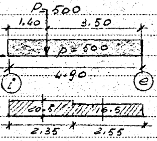
Solaio C 0.55×600×5.70 = 1880
" E 0.52×600×5.00 = 1560
P.pr. (40×35) = 330
p = 3770

	τ	l_o	A_t	φt	Ast	$A_p\sqrt{2}$
T = 0.5 —— = 5180	4.4	1.37	8.6	φ6/22"	3.5	5.1

M	b	h	d	σf	Af	t
App. $\frac{1}{15}$ —— = 1900	40	35	32	1400	4.6	29.5
Camp. $\frac{1}{10}$ —— = 2850	(60)	35	32	1400	6.9	29.5

FABBRICATO C - 1°

Cemento tipo 500

Solaio	l	p/P	M
A Sbalzo	1.00	650 P=500	Incastro $\frac{1}{2}$ — + — = 825
A1 Sbalzo	1.70	650 P=150	Incastro $\frac{1}{2}$ — + — = 1200
B	4.90	500	App. est. Come so App. int. $\frac{1}{10}$ —— = 1200 Camp. $\frac{1}{14.5}$ —— = 830
C	4.90	500	App. int. $\frac{1}{10}$ —— = 1200 Camp. $\frac{1}{13.5}$ —— = 890
D	5.90	500/500	 T_e = 0.37 — + 150 = 1240 T_i = 063 — + 350 = 2210 M App. int. = 2300 Camp. 1500×1.2 = 1860
E	4.90	500/500	 P = 500 1.40 3.50 p = 500 4.90 i e 20.5 16.5 2.35 2.55 M_i = T_i = T_e = M h App. int. = 2300 20.5 Camp. 810×1,3 = 1050 16.5

e 2° solaio

$\sigma = 50/1400$ $n = 10$

h	d	σ_f	A_f cm^2/m	t
16	14.5	1400	4.4	13.4
16	14.5	1400	6.5	13.2
aio A	e A_1			
20,5	18,5	1400	4.95	17.4
20.5	18.5	1400	3.45	17.4
20.5	18.5	1400	4.95	17.4
20,5	18,5	1400	3.7	17.2

$m_i' = \frac{1}{8} — + — = 2700$

si assume $M_i = 2300$

$x_e = 2.49$

$M_m = 1550$

h	d	σ_f	R	σ_c	μ		t
20.5	18.5	1400	28	50	$\begin{cases}0.24\\0.52\end{cases}$	$\begin{cases}4.65\\9.65\end{cases}$	17.1
20,5	185	1400	—	—	—	7.90	16.9

= 2300 (vedi sol. D)

.69 — + 360 = 2050

0.31 — + 140 = 900

$x_e = 1.80$

$M_m = 810$

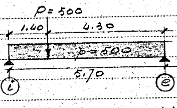
d	σ_f	R	σ_c	μ		t
185	1400	28	50	$\begin{cases}0.24\\0.52\end{cases}$	$\begin{cases}4.65\\9.65\end{cases}$	17.1
15	1400	—	—	—	5.45	13.7

./.

./.

solaio	l	p/P	M	h	d	σ_f	A_f cm^2/m	t
F	4.90	500	Comp. $\frac{1}{10}$ — = 1200	20.5	8.5	1400	5.00	17.2
G sbalzo	0.80	650/150	Incastro $\frac{1}{2}$ — + — = 330	10.0	8.5	1400	3.02	7.8
H	5.70	500/500	App. int. $\frac{1}{10}$ — = 1620 Comp. $\frac{1}{13.5}$ — = 1200	20.5 20.5	18.5 18.5	1400 1400	6.85 5.00	17.00 17.2

L 5.70 500/500



(non continuo)

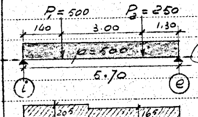
$T_i = 0.5 — + 380 = 1805$

$T_e = 0.5 — + 120 = 1545$

$x_e = 3.1$ $M_m = 2400$

Comp. 2400 × 0.8 = 1920 20.5 18.5 1400 8.1 16.9

I 5.70 500/{500 250}



(sbalzo G)

si assume $M_i = 2000$

$m_i' \frac{1}{8} — + 470 + 150 - 150 = 2510$

$T_i = 0.614 — + 380 + 60 = 2190$ $x_e = 2.30$

$T_e = 0.386 — + 120 + 190 = 1410$ $M_m = -330 + 1320 + 330 = 1320$

	M		h	d	σ_f	R	σ_c	A_f	t
App. int.	2000	=	20.5	18.5	1400	-	-	8.5	16.8
Comp.	1320	=	16.5	15	1260	25.2	50	8.45	13.7

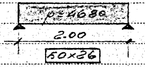
./.

Zone rinforzate

$\Delta p = 500/1$

$\Delta M = \frac{1}{10}\ 280$

Piattabande in

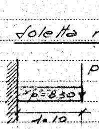


T = 0,5 — = 468

M

App. e Camp. $\frac{1}{12}$ —

Soletta rampan



M

Incastro $\frac{1}{2}$ — + —

oppure

sotto i tramezzi – Cordolo + 2 travetti

.8 = 280

× 5.7² = 900 $\Delta A_f/m = \begin{cases} 3.75 \text{ per } h = 20.5 \\ 4.7 \text{ " } h = 16.5 \end{cases}$

terne

p) T_i solaio D = 2210
T_i " E e I = 2190
P.pr. (50×26) = 280
p = 4680

	τ	l_0	A_t	st.	$A_{st.}$	$A_p\sqrt{2}$
0	4.5	1.00	7.95	φ6/17	3.3	4.65

	b	h	d	σ_f	A_f	t
= 1550	50	26	23	1400	5.3	20.9

SCALA

te a sbalzo.

p) sovracc. acc. = 350
gradini in tufo = 230
P.pr. 10 cm. = 250
p = 830

	h	d	σ_f	A_f/m	t
= 670	12	10.5	1400	5.00	9.5
	13.5	12.0	1400	4.35	11.0

./.

./.

Pianerottoli

l = 3.00   p = 830   h = 12

	M	h	d	σ_f	A_f/m	t
App.	$\frac{1}{12}$ — = 620	12	10.5	1400	4.6	9.6
Camp.	$\frac{1}{15}$ — = 500	12	10.5	1400	3.75	9.6

Pianerottoli a sbalzo

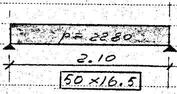
1) l = 1.40   p = 830   P = 150

	M	h	d	σ_f	R	σ_c	μ	A_f/m	t
Incastro	$\frac{1}{2}$ — + — = 1020	13.5	12	1350	27	50	$\begin{cases} 0.32 \\ 0.58 \end{cases}$	$\begin{cases} 3.85 \\ 6.95 \end{cases}$	10.8

2°) l = 1.30

	M	h	d	σ_f				A_f/m	t
Incastro	$\frac{1}{2}$ — + — = 900	13.5	12	1400	–	–	–	5.95	10.8

Trave in spessore di solaio (esterno)



p = solaio I (Te) = 1410
Balcone G (0.8×65×150) = 670
P.pr. (50×16.5) = 200

T = 0.5 — = 2390 τ –

	M	b	h	d	σ_f	R	σ_c	μ	A_f	t
App. e Camp.	$\frac{1}{12}$ — = 840	50	165	145	1350	27	50	$\begin{cases} 0.64 \\ 0.65 \end{cases}$	$\begin{cases} 4.65 \\ 4.70 \end{cases}$	12.8

FABBRICATO C – Copertura

Cemento tipo 500 $\sigma = 50/1400$ $n = 10$

Solaio	l	p	M	h	d	σf	Af/m	t
A	4.90	600	p/ P. pr. = 220 Sovracc. = 200 Par. pendenze = 180 p = 600					
			App. verso B $\frac{1}{10}$ — = 1440	35.0	33	1400	3.25	31.5
			App. verso C $\frac{1}{10}$ — = 2020	35	33	1400	4.64	31.3
			Comp. $\frac{1}{13}$ — = 1110	35	33	1400	2.51	31.5
B	4.90	600	App. verso A — = 1440	35.0	33	1400	3.25	31.5
			Comp. $\frac{1}{14}$ — = 1030	35.0	33	1400	2.34	31.5
C	5.80	600	App. int. $\frac{1}{10}$ — = 2020	35	33	1400	4.64	31.3
			Comp. $\frac{1}{13}$ — = 1550	35	33	1400	3.55	31.3
D	4.90	600	Comp. $\frac{1}{9}$ — = 1600	35	33	1400	3.66	31.3
E	5.70	600	Comp. $\frac{1}{13}$ — = 1550	35	33	1400	3.55	31.3
F	5.70	600	Comp. $\frac{1}{9}$ — = 2160	35	33	1400	4.96	31.3
G sbalzo	1.00	600/200	Incastro $\frac{1}{2}$ — + — = 500	11	9.5	1400	4.1	8.7

·/.

Travi

Trave a

l = 1.70
40×35

Solaio E 0.55 × 600 × 5.70 = 1880
Solaio C 0.55 × 600 × 5.80 = 1920
P. pr. (40 × 35) = 330
p = 4130

T = 0.5 — = 3500 τ

M	b	h	d	σf	Af	t
App. $\frac{1}{14}$ — = 850	40	35	32	1400	2.03	29.9
Comp. $\frac{1}{10}$ — = 1170	40	35	32	1400	2.8	29.9

Trave b

l = 1.87
40×35

Solaio E 0.45 × 600 × 5.70 = 1540
Muro = 200
P. pr. (40 × 35) = 330
p = 2070

T = 0.5 — = 1940 τ

M	b	h	d	σf	Af	t
App. $\frac{1}{14}$ — = 520	40	35	32	1400	1.23	30.1
Comp. $\frac{1}{10}$ — = 730	40	35	32	1400	1.73	30.1

Solaio Copertura scala

l = 2.85 p. = 400

M	h	d	σf	Af	t
Comp. $\frac{1}{8}$ — = 410	16.5	15	1400	2.06	14.2