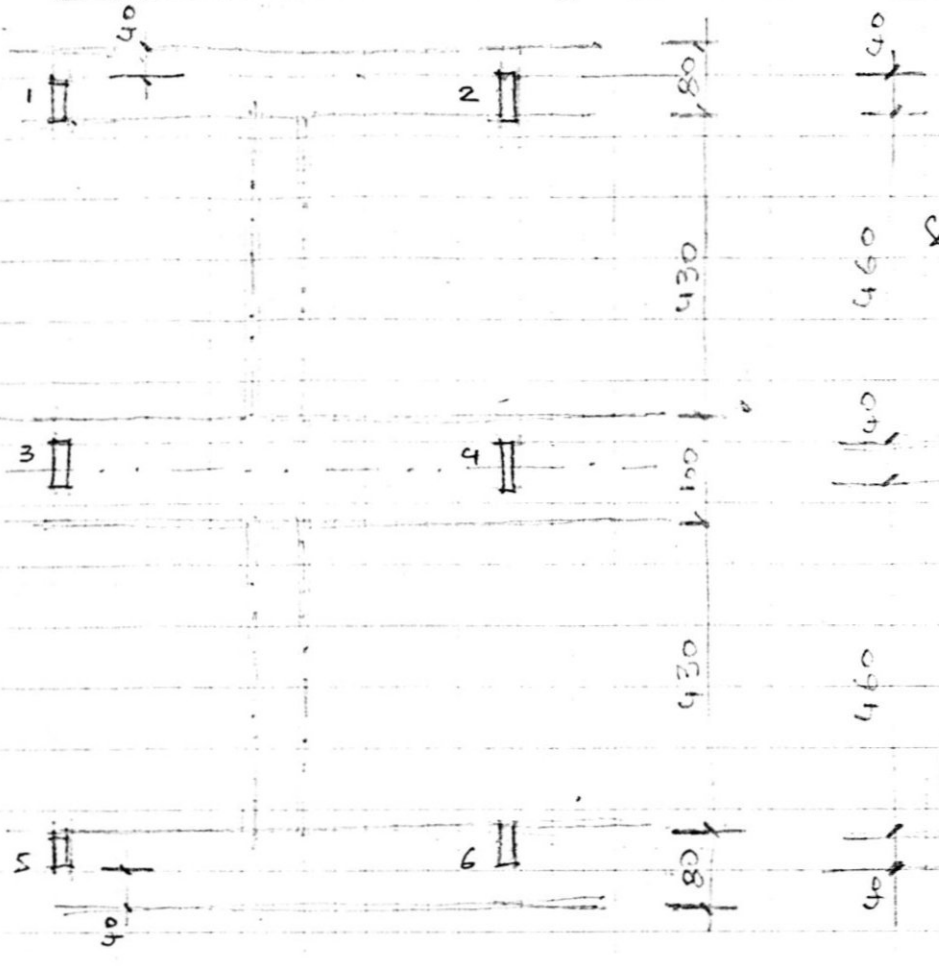
I.A.C.P - BRINDISI - LEGGE 457/78 - N°60 ALLOGGI IN FRANCAVILLA FONTANA -
COSTRUZIONE N° 60 BOX — RELAZIONE DI CALCOLO —

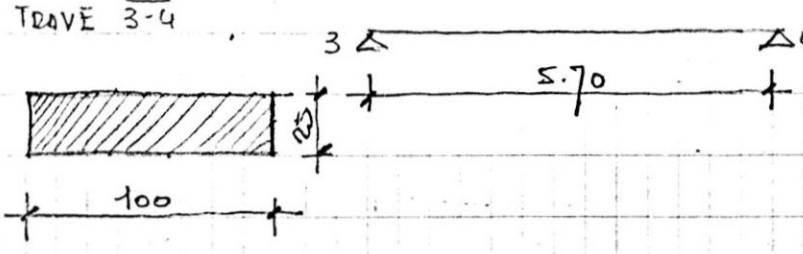


- ANALISI DEI CARICHI -

Solaio (h=20+5)

p.p	= 295,00 Kg/
impermeabilizzazione	= 75,00 "
intonaco	= 30,00 "
Solaccazzo	= 150,00 "
	550,00 K

TRAVE 3-4



p.p = 1,00 × 0,25 × 1,00 × 2500 = 625,00 Kg/
Solaio = 550 × 4,80 = 2640,00
3265,00 Kg/

$M = \frac{q l^2}{12} = \frac{3265 \times \overline{5,70}^2}{12} = 8840$ Kgm ; $\sqrt{8840} = 94,02$

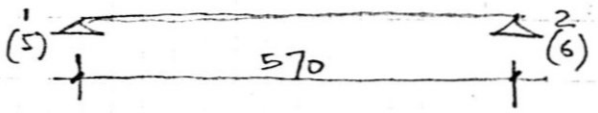
$C_m = \frac{22}{94,02} = 0,234 \quad \frac{V_f = 2200}{A_f = A'_f} \rightarrow \sigma_c \simeq 72$ Kg/cmq

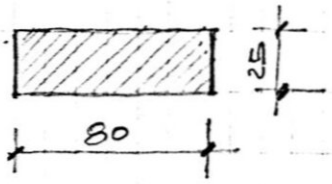
$A_f = A'_f = 0,00212 \times 94,02 \times 100 = 19,93$ cmq → 10 + 10 ϕ16 = 20,11 cmq
staffe ϕ8 a 4br/20"

$R_{3/4} = R_{4/3} = 3265 \times \frac{5,70}{2} = 9305$ Kg.

$R_3 = R_4 = 9305 \times 2 = 18610$ Kg.

TRAVI (1-2)-(5-6)





p.p = 0,80 × 0,25 × 1,00 × 2500 = 500,00 kg/m
Solaio = 550 × 5,60/2 = 1540,00 "
Tramezzi = 500,60 "
2540,00 kg/m

$H = \frac{ql^2}{12} = 2540 \times \frac{5,70^2}{12} = 6877$ kgm

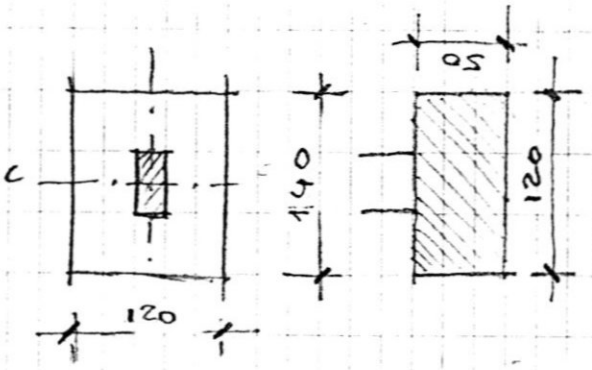
$\sqrt{M/b} = \sqrt{\frac{6877}{0,80}} = 92,71$; $c_h = \frac{22}{92,70} = 0,237$ → $\sigma_c \simeq 70$ kg/cm²

$A_f = A'_f = 0,00210 \times 92,71 \times 80 = 15,57$ cm². → 8+8 ϕ16 = 16,08 cm²
staffe ϕ8 a 4 br/20"

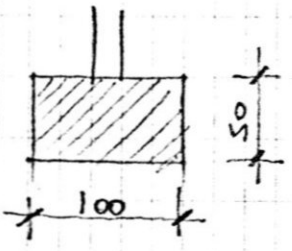
$R_{1/2} = R_{2/1} = R_{5/6} = R_{6/5} = 2540 \times \frac{5,70}{2} = 7240$ kg

$R_1 = R_2 = R_3 = R_4 = 7240 \times 2 = 14480$ kg

- Plinti -



p.p = 1,00 × 1,20 × 0,50 × 2500 = 1500 kg
p.p. pilastro = 0,20 × 0,40 × 3,00 × 2500 = 600 "



- Punzonamento:

$\tau = \frac{19.000}{2 \times (20+40) \times 50} = 3,17$ kg/cm²

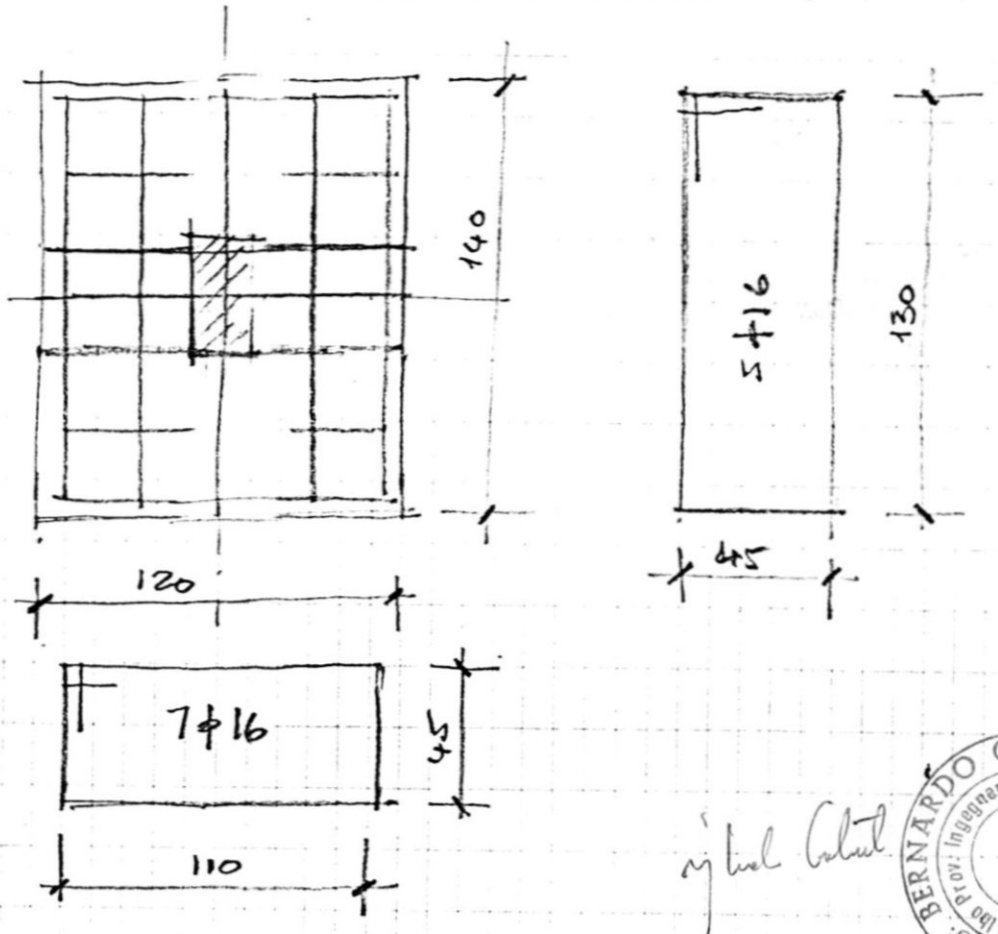
$k_t = \frac{21.000}{120 \times 140} = 1,25$ kg/cm²

$M_{c-c} = 12500 \times \frac{1,40^2}{2} = 12.250$ Kgm

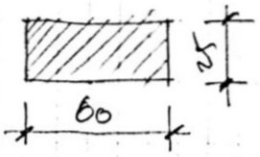
$\sqrt{M/b} = \sqrt{\frac{12250}{1,40}} = 93,54$

$c_\eta = \frac{45}{93,54} = 0,48$ $\xrightarrow{\frac{K_f = 2200}{A_f = A'_f}}$ $\sigma_c \simeq 40$ kg/cmq

$A_f = A'_f = 0,00109 \times 93,54 \times 140 = 14,27$ cmq $\rightarrow$ 7+7 ϕ 16 -



TRAVE 1-3-5



p.p = $0,60 \times 0,25 \times 1,00 \times 2500$ = 375 kg/ml
muretto = 500 "
875 kg/ml

$M = \frac{pl^2}{8} = 875 \times \frac{5,20^2}{8} = 2957$ Kgm ; $\sqrt{M/b} = 70,20$

$c_\eta = \frac{22}{70,20} = 0,313$ $\xrightarrow{\frac{K_f = 2200}{A_f = A'_f}}$ $\sigma_c \simeq 55$ kg/cmq ; $A_f = A'_f = 0,00156 \times 70,20 \times 60 = 6,57$ cmq

4+4 ϕ 16 = 8,04 cmq

- CARATTERISTICHE DEI MATERIALI -

CALCESTRUZZO - CLASSE 250

ACCIAIO - FeB38K

IL CALCOLATORE

Dott. Ing. BERNARDO COSTANTINO - Ord. prov. Ingegneri di Brindisi - N. 190

19 FEB 1986

VISTO - IST. AUTONOMO CASE POPOLARI per la Prov. di BRINDISI

IL DIRETTORE DEI LAVORI
(*Dott. Ing. Antonio Longo*)